

Application of Computational Fluid Dynamics to Optimization of Hydraulic Design and Structure Operation in Everglades Restoration Projects

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Background

- Great deal of hydraulic designs are carried out in support of the Everglades Restoration Projects. Reduced-scale physical models typically implemented: reliable but costly.
- Computational Fluid Dynamics (CFD): Evaluate and optimize hydraulic performance and design of hydraulic structures in Everglades Restoration projects

Governing equations, NS :

$$\frac{\partial u_j}{\partial x_j} = q$$

$$\frac{\partial(\rho u_i)}{\partial t} + \frac{\partial}{\partial x_j}(\rho u_i u_j) = \frac{\partial}{\partial x_j}[\mu_e(\frac{\partial u_j}{\partial x_i} + \frac{\partial u_i}{\partial x_j})] - \rho g \frac{\partial \zeta}{\partial x_i} + F_i$$

Turbulence model: k-ε closure

$$\frac{\partial(\rho k)}{\partial t} + \frac{\partial}{\partial x_j}(\rho u_j k) = \frac{\partial}{\partial x_j}(\frac{\mu_e}{\sigma_k} \frac{\partial k}{\partial x_j}) + G_k - \rho \varepsilon$$

Commercial CFD-software
package ANSYS FLUENT

$$\frac{\partial(\rho \varepsilon)}{\partial t} + \frac{\partial}{\partial x_j}(\rho u_j \varepsilon) = \frac{\partial}{\partial x_j}(\frac{\mu_e}{\sigma_\varepsilon} \frac{\partial \varepsilon}{\partial x_j}) + \frac{\varepsilon}{k}(C_1 G_k - C_2 \rho \varepsilon)$$

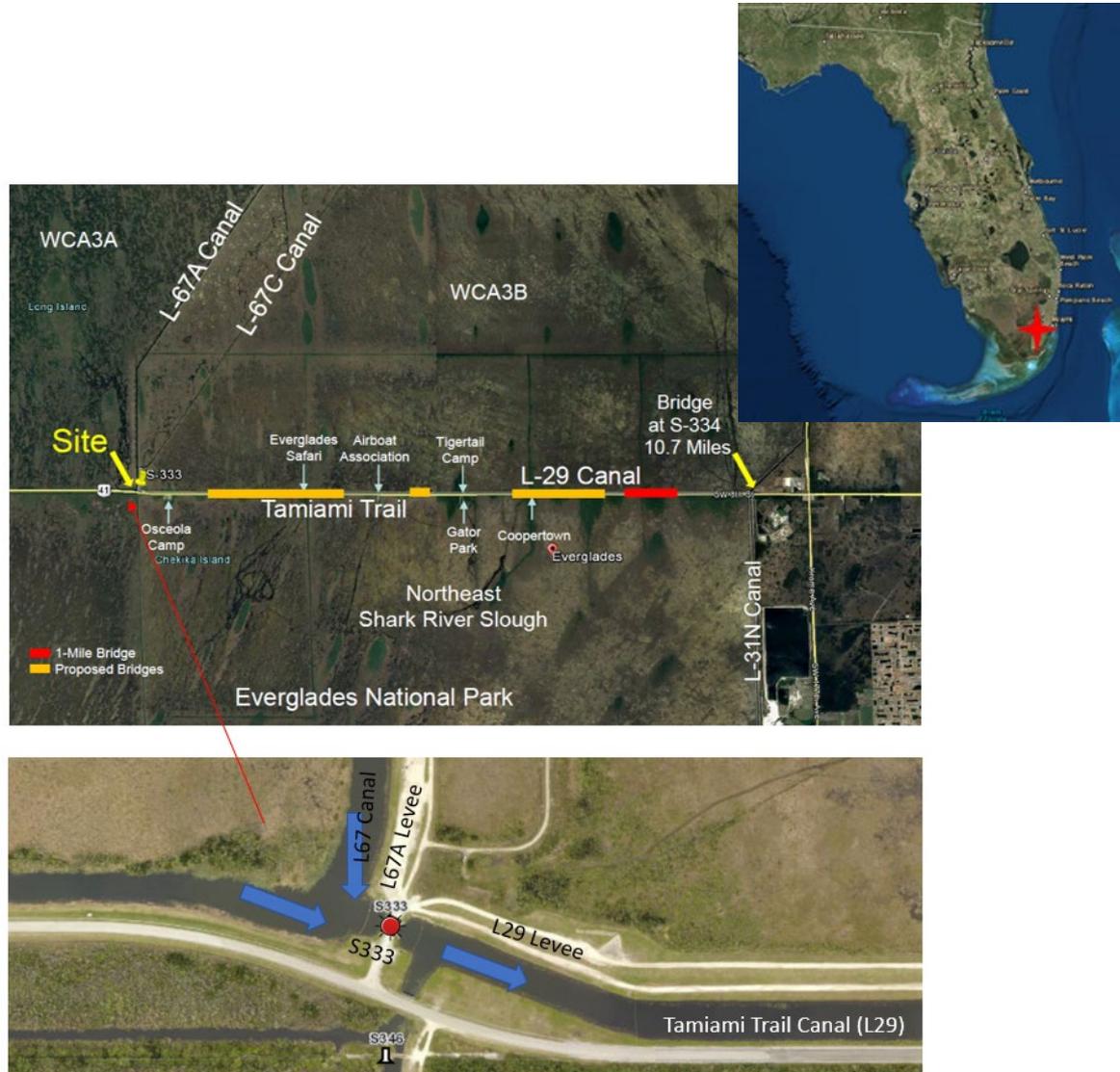
Case Study I:

S333N Spillway Design, Layout and Impact Assessment

- S333 is a trapezoidal-sill reinforced concrete spillway, located at the intersection of L-29 and L67 canals
- Proposed new S333N spillway to accommodate additional discharge

Objective:

Determine the layout, the design, operation criteria, and impact of a newly proposed spillway



Case Study I:

S333N Spillway Design, Layout and Impact Assessment

S333N Sizing

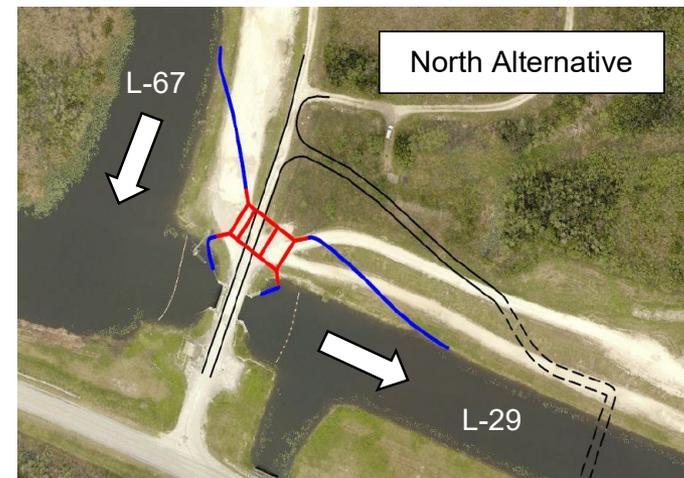
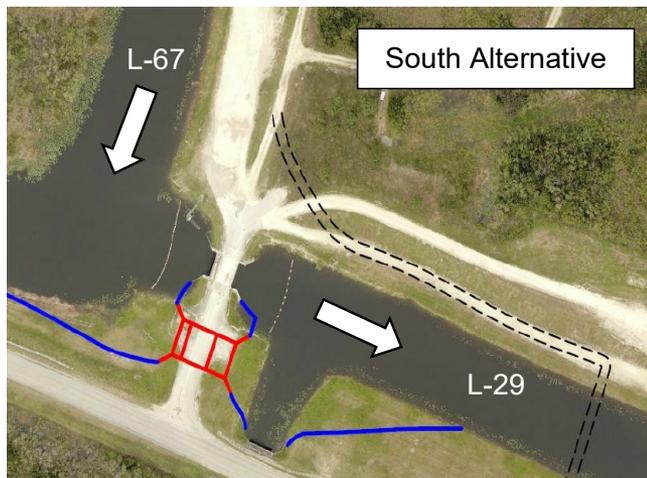
S333 Capacity: 1,350 cfs, One gate 29 ft wide

$$\frac{y_c}{G_0} = a \left(\frac{H-h}{G_0} \right)^b \quad y_c = \frac{Q^{2/3}}{L^{2/3} g^{1/3}} \quad \frac{h}{G_0} \geq 1.0$$

S333N Proposed Capacity: 1,150 cfs, Two gates each 14 ft wide

S333N Required gate opening: 2 x 6.40 ft (at design HW of 9.5 ft-NGVD, and TW of 9.0 ft-NGVD)

Layout Alternatives

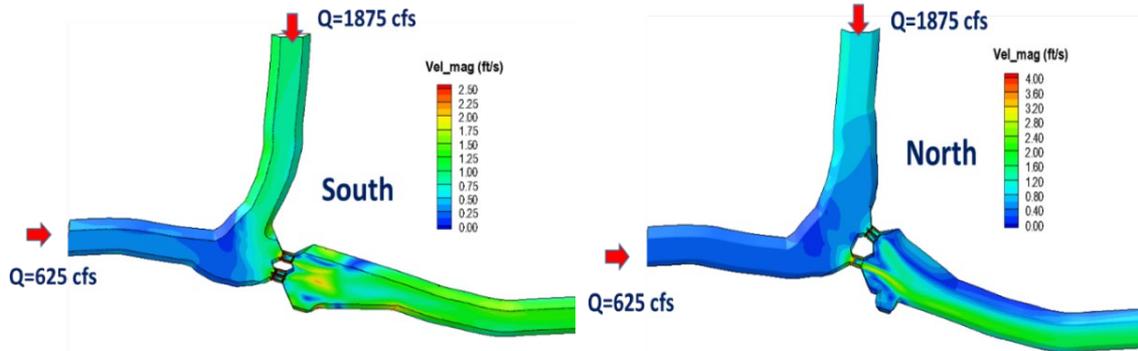


Case Study I: S333N Spillway Design, Layout and Impact Assessment

Flow Scenario A: 75% flow from L-67

H=9.5 ft, T=9.0 ft

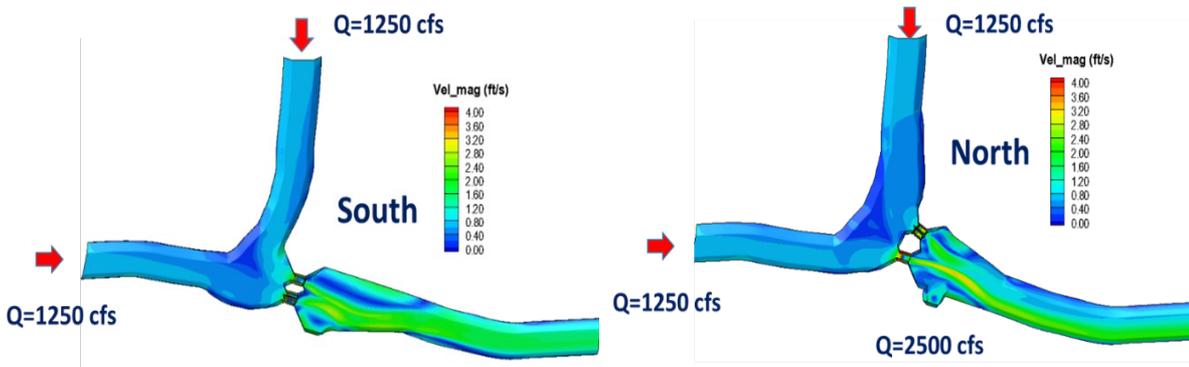
Near Bed Velocity Contours



South alt: 1.0-2.0 ft/s
North alt: 1.0-3.0 ft/s

Limestone layer:
scouring not likely

Flow Scenario B: 50% flow from L-67



South alt: 1.5-3.2 ft/s
North alt: 1.8-3.2 ft/s

Eddy formation
downstream,
Flow bias towards east
bank in L-29 Canal

Place proposed S333N structure north of the existing structure S333 at angle 25-30 degrees with S333

Case Study I:

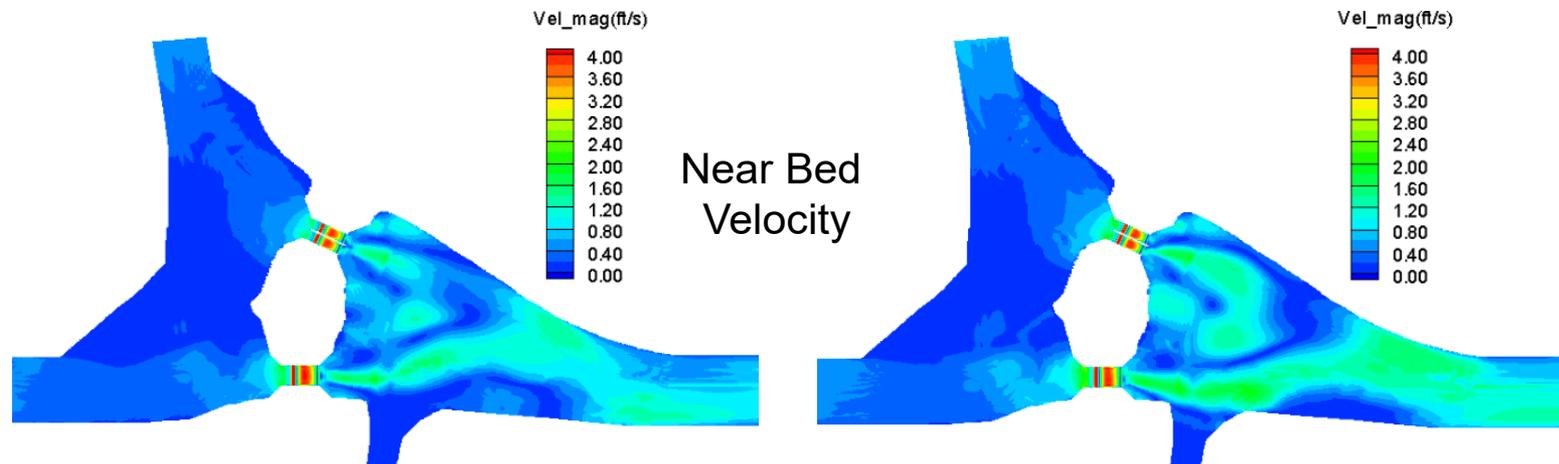
S333N Spillway Design, Layout and Impact Assessment

Further Analysis

- Extreme scenarios analysis: high flows + low tailwater

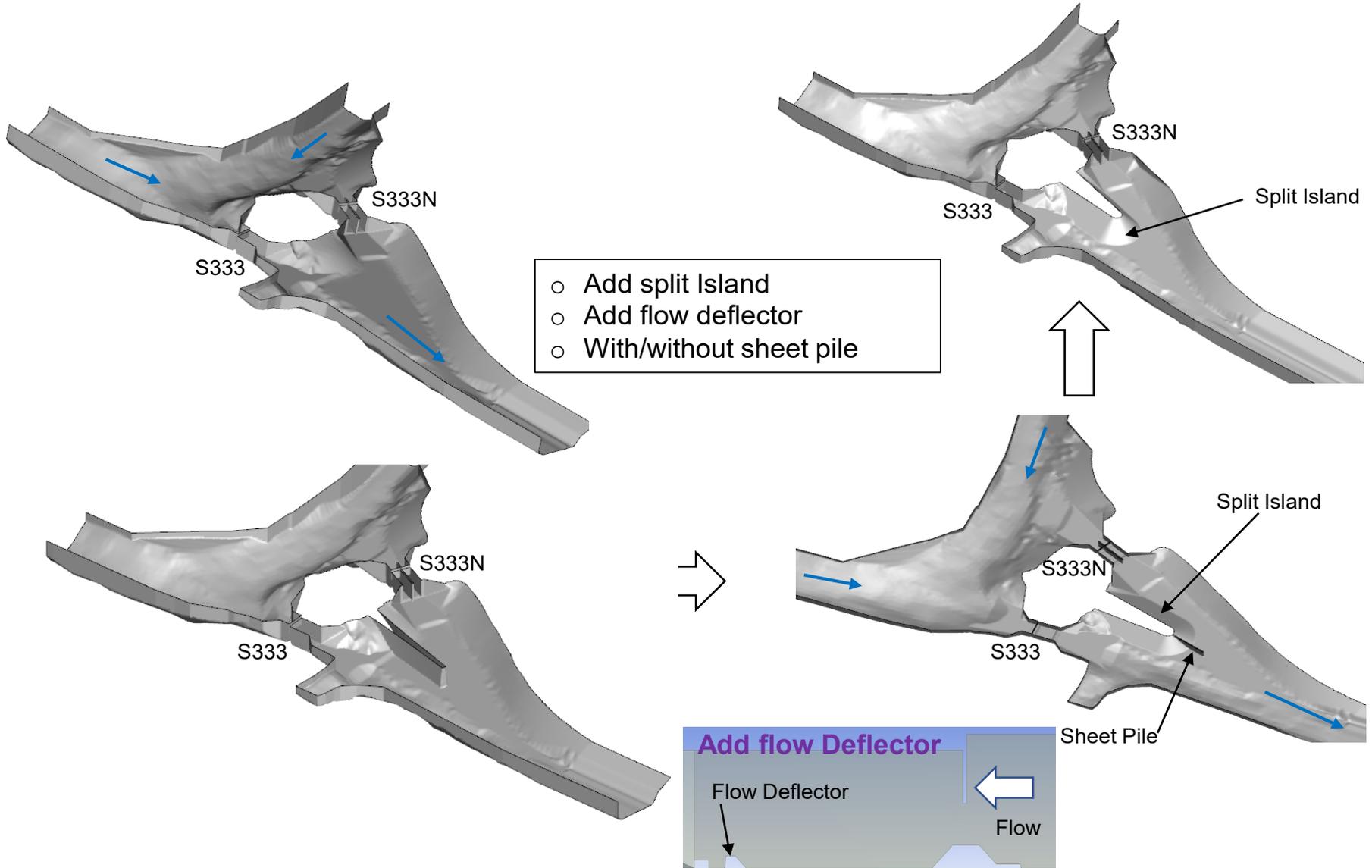
H=10.5 ft, T=8.5 ft,
Q₃₃₃=1,350 cfs, Q_{333N}=1,150 cfs

H=10.5 ft, T=8.5 ft,
Q₃₃₃=1,620 cfs, Q_{333N}=1,380 cfs



- With the adjusted angle of S333N spillway, flow jets are evenly distributed at downstream, without any severe potentials of eddy formation or scouring
- As conditions became extreme, flow jet downstream of the structures began to oscillates between north and south bank of L-29 Canal

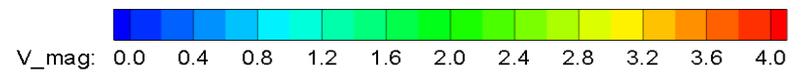
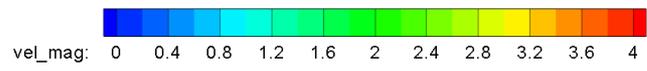
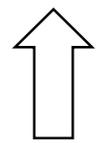
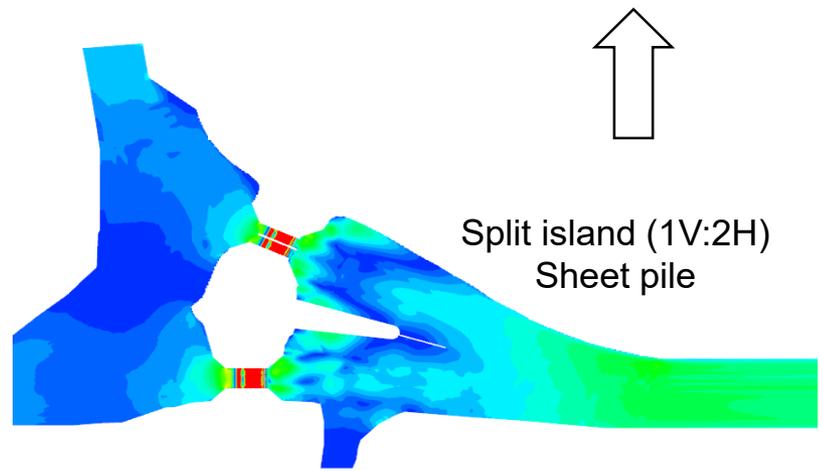
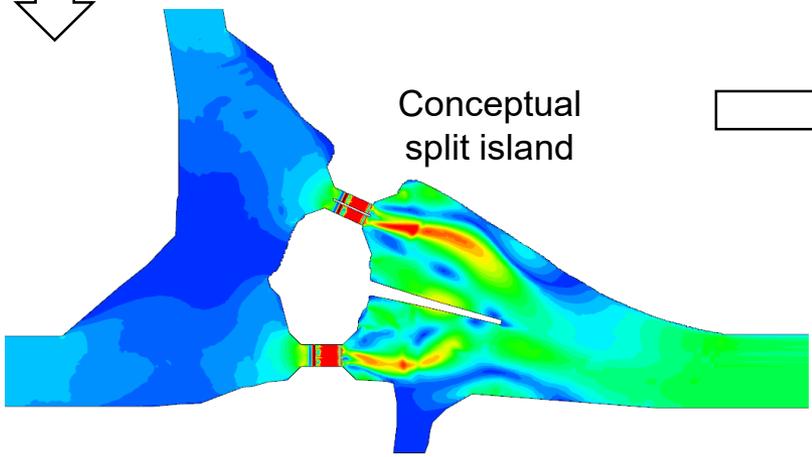
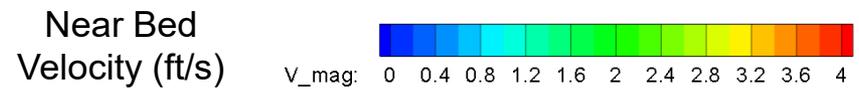
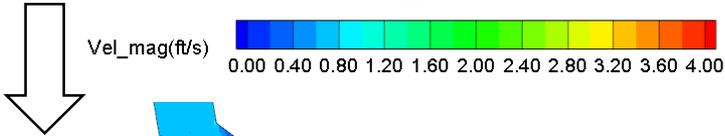
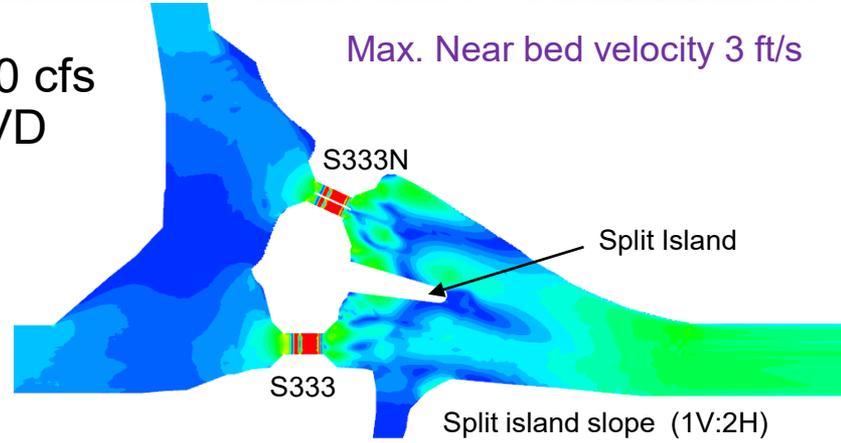
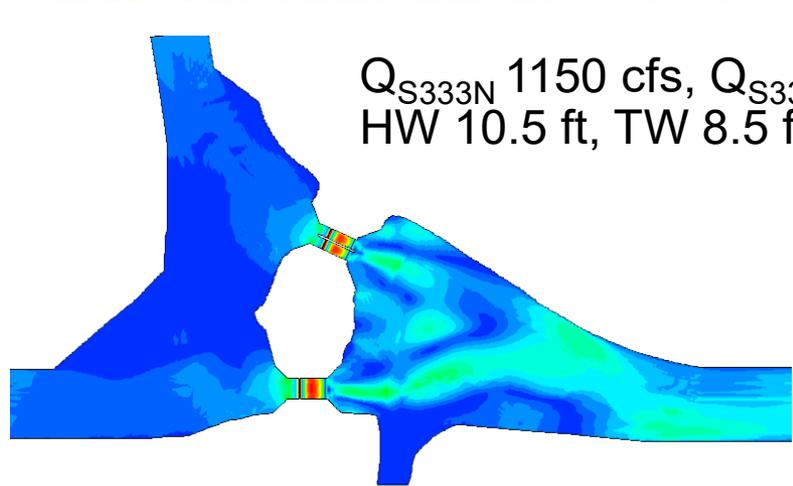
Design Optimization



Near Bed Flow Field

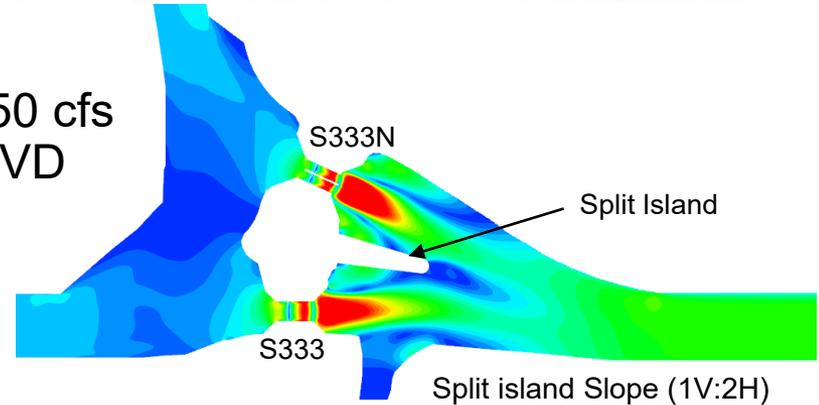
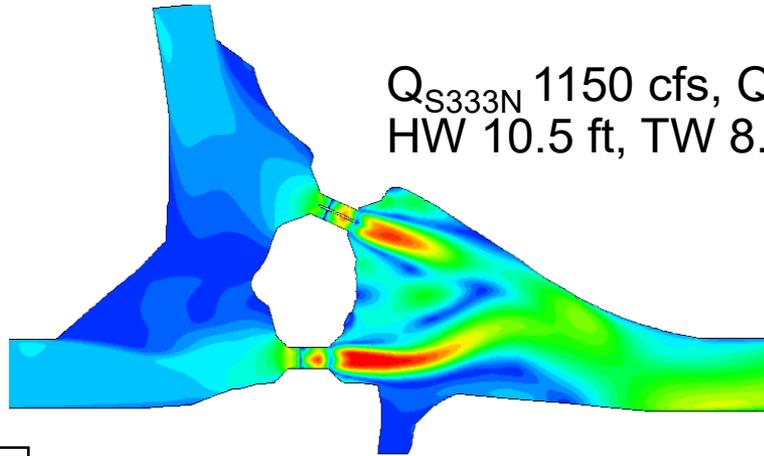
Q_{S333N} 1150 cfs, Q_{S333} 1350 cfs
 HW 10.5 ft, TW 8.5 ft-NGVD

Max. Near bed velocity 3 ft/s



Surface Flow Field

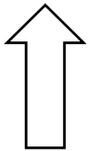
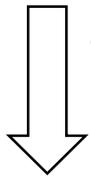
Q_{S333N} 1150 cfs, Q_{S333} 1350 cfs
HW 10.5 ft, TW 8.5 ft-NGVD



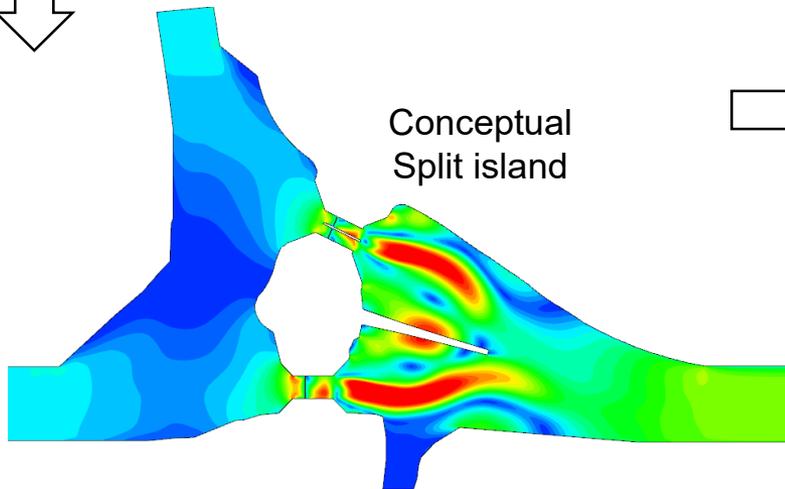
V_mag: 0 0.4 0.8 1.2 1.6 2 2.4 2.8 3.2 3.6 4

Vel_mag(ft/s)
0.00 0.40 0.80 1.20 1.60 2.00 2.40 2.80 3.20 3.60 4.00

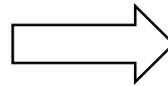
Surface Level
Velocity (ft/s)



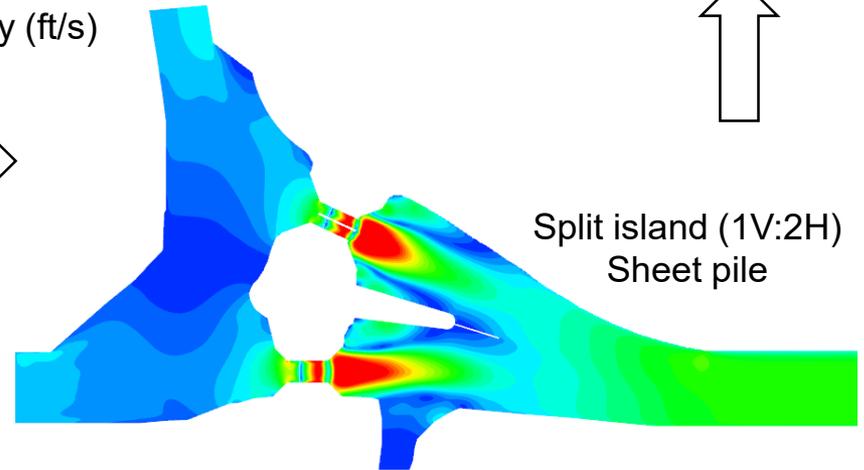
Conceptual
Split island



vel_mag: 0 0.4 0.8 1.2 1.6 2 2.4 2.8 3.2 3.6 4



Split island (1V:2H)
Sheet pile



V_mag: 0.0 0.4 0.8 1.2 1.6 2.0 2.4 2.8 3.2 3.6 4.0

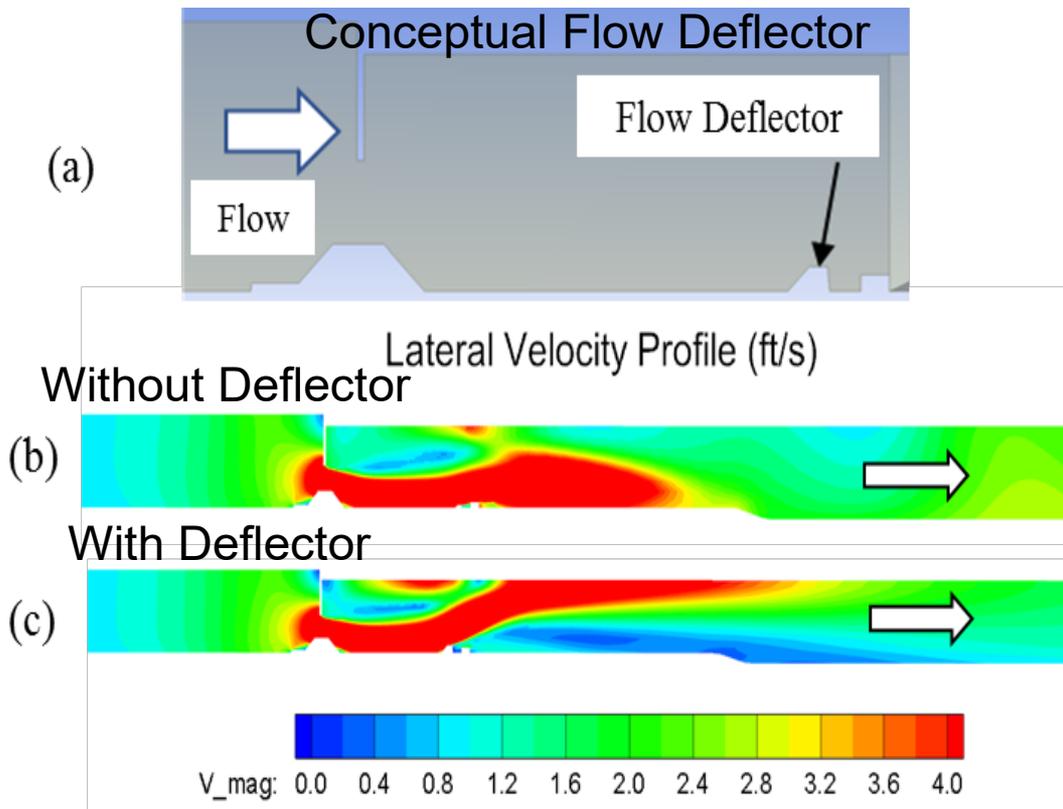
Case Study I:

S333N Spillway Design, Layout and Impact Assessment

$H=10.5$ ft, $T=8.5$ ft,
 $Q_{333}=1,350$ cfs, $Q_{333N}=1,150$ cfs

Design Improvements

- Installation of flow deflectors at both end-sills, raised by 1.5-2 ft



- Flow jet travels longer and expands slower for energy dissipation without the flow deflector
- Deflector directs the discharge upwards, reduces near bed velocities
- Near bed velocities significantly reduce from 4 ft/s to 2.5 ft/s
- Reduces riprap protection requirements in L-29 canal

Case Study II:

S332B and C Pump Stations Refurbishment Design

- S332B/C pump stations are located south of Pump Station 331, along the L-31N canal
- Construction did not adhere to District standards, meant to be temporary, Frequent repair works
- Inflow canal leading to the pump is oriented at 90° with the pump: flow field biased

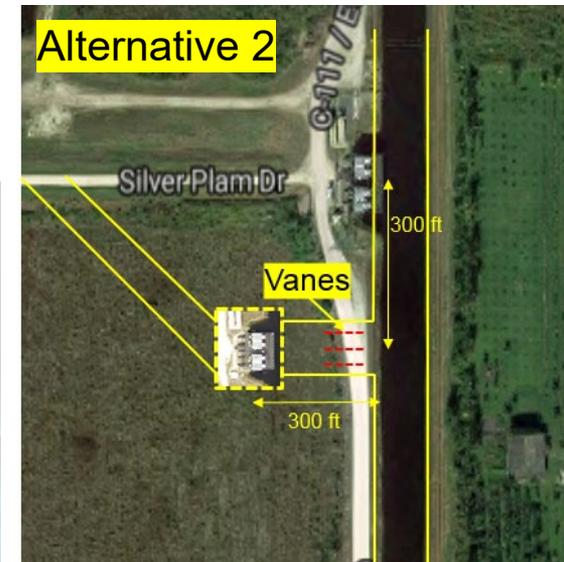
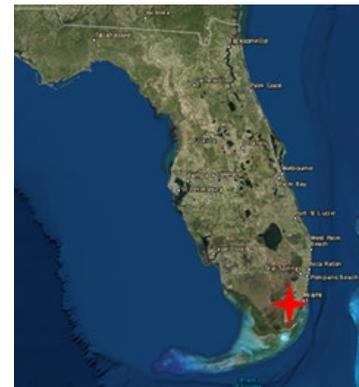
Objective:

Apply CFD model to optimize the refurbishment of S332B/C Pump Stations for improving hydrodynamic performance

S332B Layout Alternatives



- 1) Move pump Downstream
- 2) Add vanes
- 3) Move pump further west

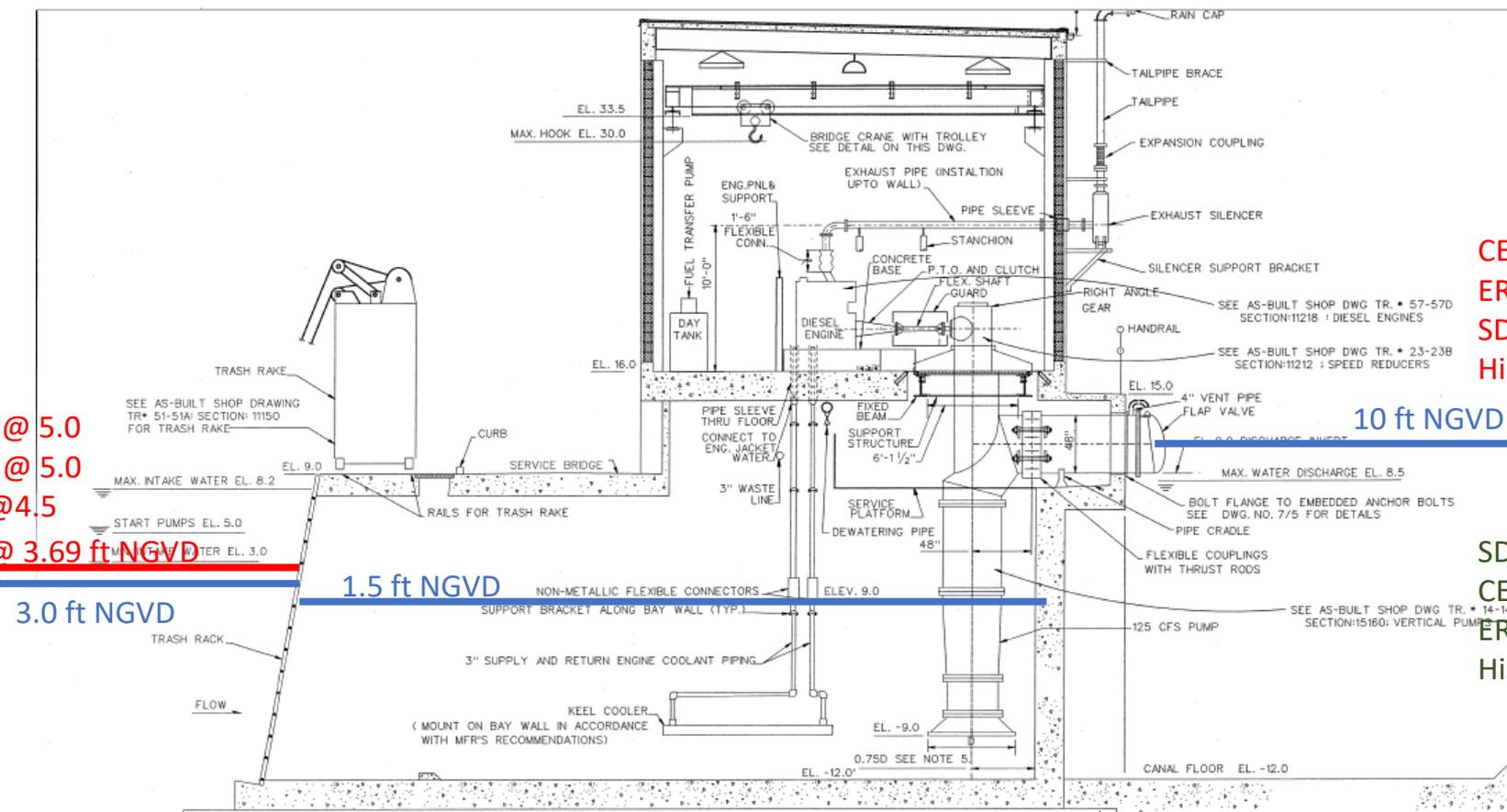


Case Study II: S332B and C Pump Stations Refurbishment Design

ERTP @ 5.0
 CEPP @ 5.0
 SD I @ 4.5
 Hist @ 3.69 ft NGVD

CEPP @ 8.39
 ERTP @ 8.30
 SD I @ 8.22
 Hist @ 9.88

SDI @ 7.80
 CEPP @ 7.67
 ERTP @ 7.30
 Hist @ 9.45

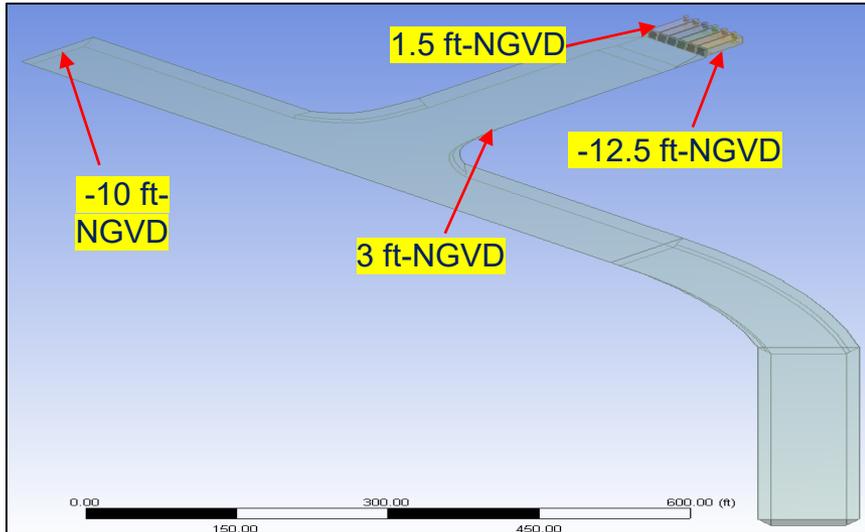


S332B

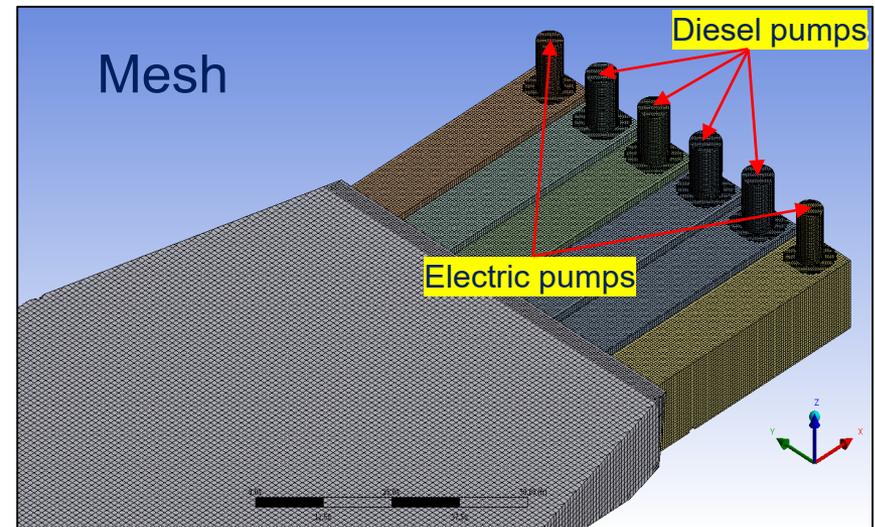
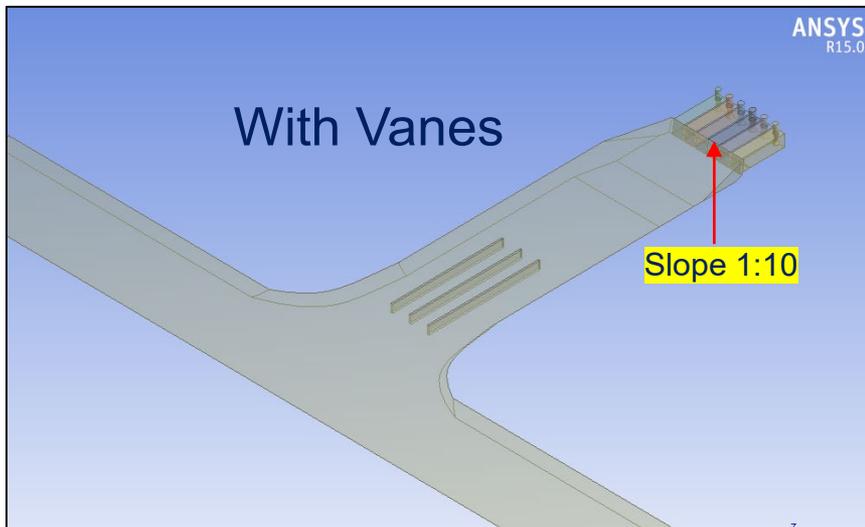
Flow simulation uses H=3.00 ft, T=10.0 ft NGVD

Case Study II:

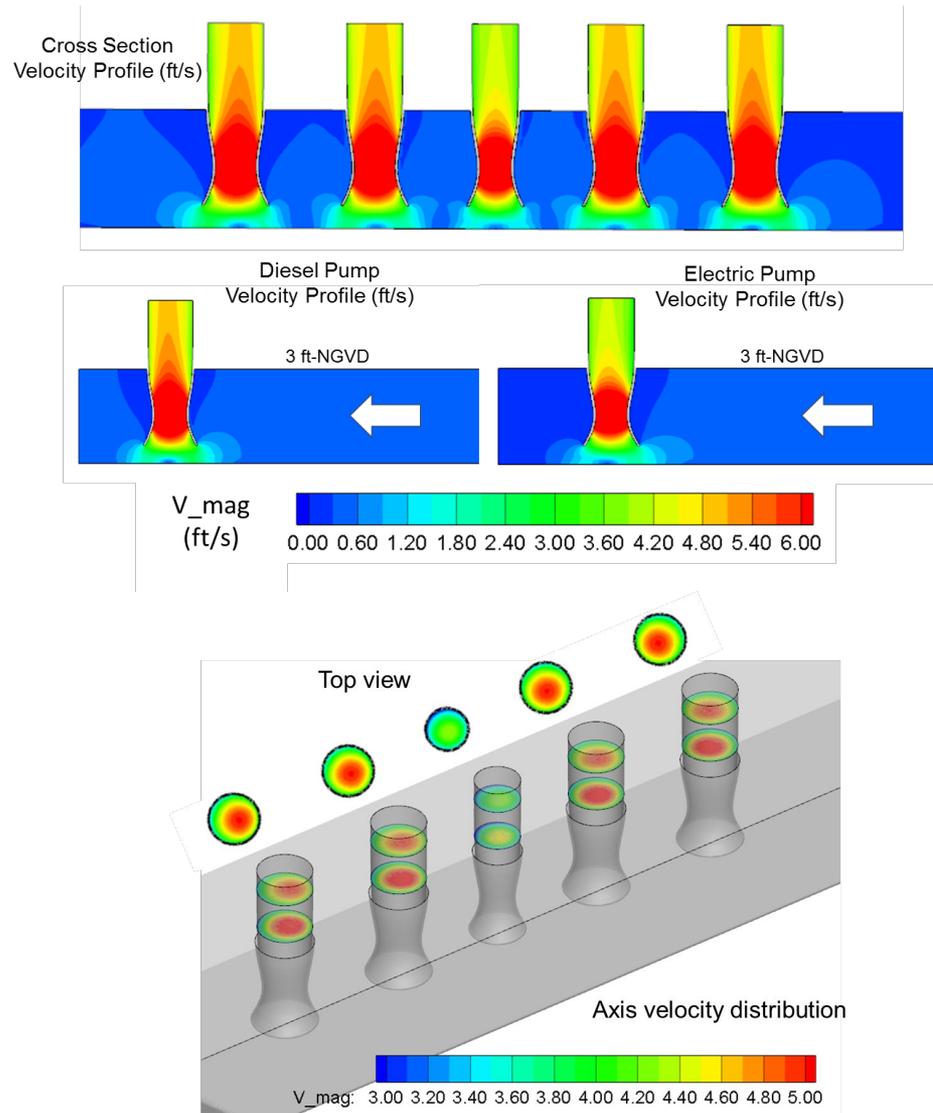
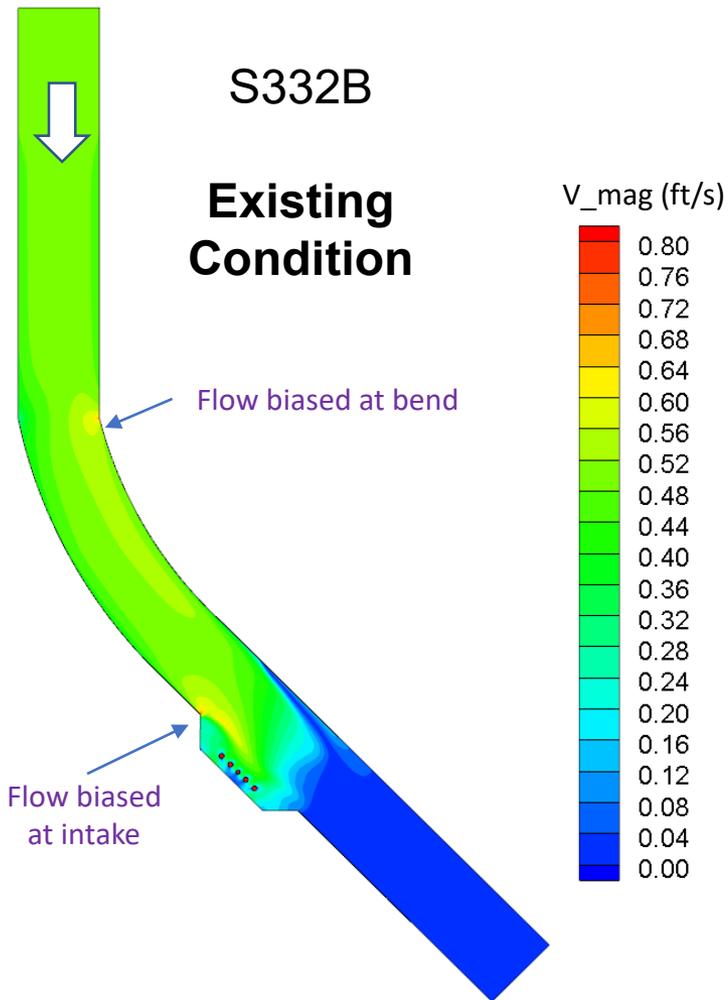
S332B and C Pump Stations Refurbishment Design



- Canal bottom @ -10 ft-NGVD based on as-builts
- Forebay extended 50 ft
- Slope 1:10 near forebay
- 4 Diesel pumps (125 cfs)
- 2 Electric pumps (75 cfs)
- Design Capacity 650 cfs, the bottom elevation is -12.5 ft NGVD

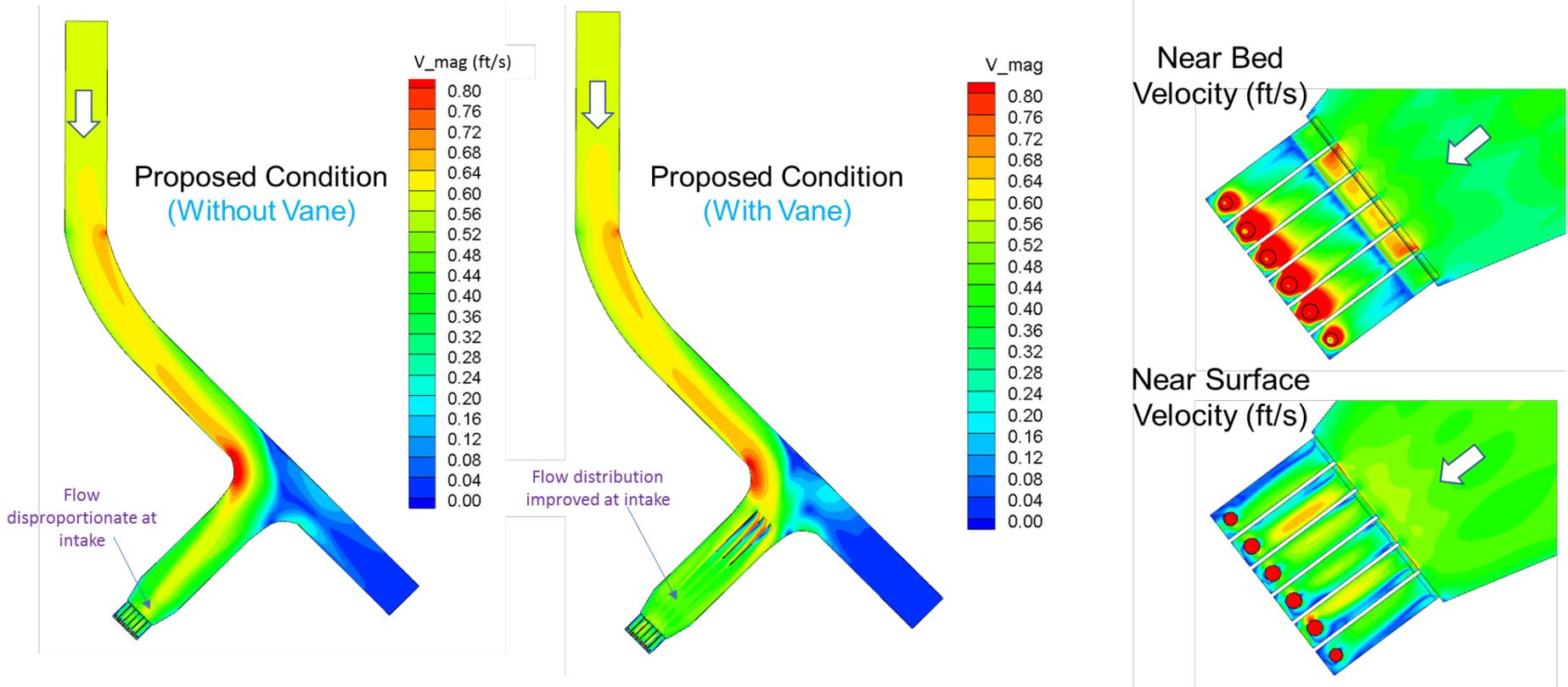


Case Study II: S332B and C Pump Stations Refurbishment Design



Case Study II: S332B and C Pump Stations Refurbishment Design

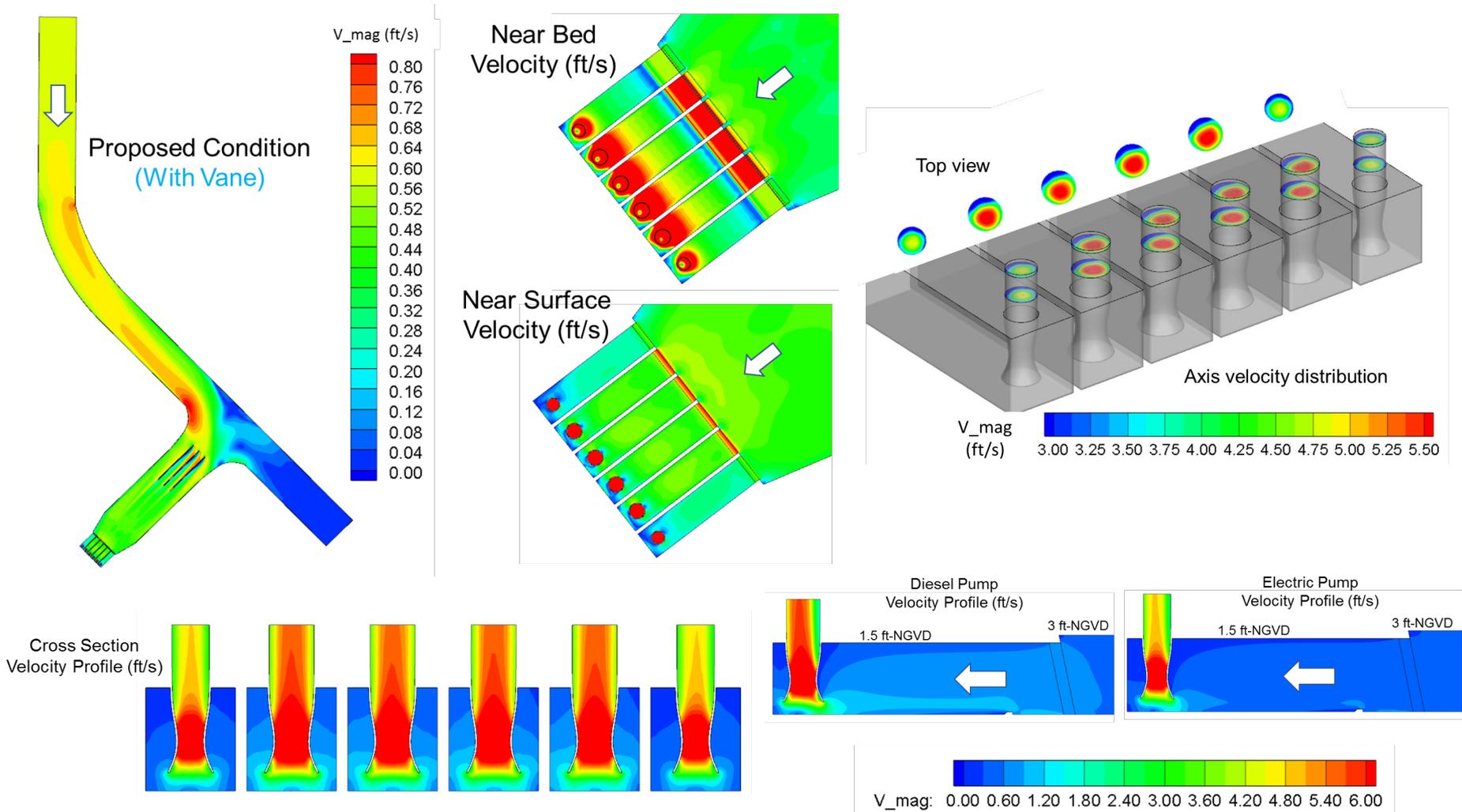
Proposed Condition Simulation with and Without Vanes



Flow vane improves pump approach flow distributions

Case Study II: S332B and C Pump Stations Refurbishment Design

Proposed Condition: Simulation with Vanes and Trash Rack



Summary

- CFD successfully applied to hydraulic analysis of two water control structures in Everglades Restoration Projects
- CFD is used as a complement or alternative to physical model and prototype results
- CFD was systematically used to:
 - Evaluate structure performance and design
 - Predict flow behavior and operation risk
 - Optimize structure design